

An expansion of
MIDWEST PAVEMENT MANAGEMENT, INC.

1404 Concordia Avenue, St. Paul, MN 55104 — 612 / 644-2996
FAX — 612 / 644-1045



Quality Services Since 1957

C G Kruse P.E., President
Eugene L Skok, Jr., Ph.D.
Director of Research
Erlend Lukinen, P.E.
Director of Engineering
Robert L. Orthmeyer, P.E.
General Manager

April 27, 1990

MEMO TO: Mr. ^{Graig} Greg Gilbertson
Soils Engineer District 2A
Mn/DOT
Box 490
Bemidji, MN 56601

FROM: Ron Urbach *RRU*

RE: Sampling Information from the SHRP-LTPP Guide for Drilling and Sampling Version 1

This is some of the information from the Drilling and Sampling Guide that we discussed on April 26. This is not a copy of the complete Guide but it covers the areas that we were discussing. The information is to be used for all sampling and drilling on the SHRP projects. It covers the GPS and SPS sections. This information you can keep for your records and if you have any further questions, give me a call or talk to Gene Skok.

I will be in Canada the week of April 30-May 5. I will have Gene or Dick Ingberg give you a call concerning the SPS 5 information that we also discussed. I will be in Bemidji working on the SPS 3 sections mid-week of April 30.

It was nice to talk with you in Phoenix last week.

cc: Gene Skok

TRDF

MEMORANDUM

TECH MEMO NO: TM-EC-55

AUTHORS: J.L. Groeger and G.E. Elkins

DISTRIBUTION: R. Urbach, E. Lukanan, G. Skok, D. Ingberg, A. Hanna, A. Pelzner, B. Hadley

SUBJECT: Comments on Proposed Sampling Plan for SPS-5 Westbound Trunk Highway 2 West of Bemidji, MN.

DATE: May 31, 1990

FILE: PI-TM-EC-55

We have slightly revised the SPS-5 materials sampling plans you have submitted for the above referenced project. The changes we have proposed are as follows:

- Renumber the section 12 designation from 270510 to 270512. As it is presently specified, the data from the two sections currently designated 270510 will not be able to be entered into the RIMS (Regional Information Management System) because each test section must be uniquely identified (see Figure 1).
- Addition of four C-type cores. Two cores should be added before section 9 and two cores should be added after section 12. Because there is a transition of approximately a mile and a half between sections 8 and 9, there should be a way to quantify the bound layer (AC, treated base/subbase) thicknesses. Therefore cores C₂₄ and C₂₅ were added. Cores C₃₅ and C₃₆ were added to determine bound layer thicknesses at the end of the project.
- Relocation of the sampling area between section 9 and section 10. We propose to move the sampling area from near the end of section 9, to near the beginning of section 10. This will provide for more precise layer thickness information for section 10.
- One other concern are core locations C-8, C-17, C-23 and C-28 on your proposed sampling plan. We assume the Minnesota DOT would like to have six-inch cores instead of four-inch cores for their own testing purposes. If this is the case then it is fine, otherwise, we do not see the need to take a six-inch core when a four-inch core will suffice.

Overall, your sampling plans for this project were very good. The minor changes we propose are based on the methodology we are developing for material sampling and testing plans for the SPS-5 experiments. Of course, you may have to revise this further depending on actual site conditions. If possible, in the future, please submit a copy of site plans and profile sheets showing the location of the test sections. This will give us a better idea of the variability in the cross-section of the entire project and be more useful when reviewing materials sampling plans for the SPS experiments.

Please call us if you have any questions about this sampling plan.

/jg

Attachments: Letter of 5/24/90 from Ron Urbach to Gary Elkins.

SPS-5 PROJECT - 14 MILES WEST OF BEMIDJI, MN WESTBOUND TH 2

		TRAFFIC FLOW			
		50'	325'	250'	150'
Section 1			Section 2	Section 3	Section 4
C1*	C2*			C7 C8 C1	C7 C8 C1
270503			270508	270502	270509

S1 •

		TRAFFIC FLOW			
		700'	450'	350'	8400
Section 5			Section 6	Section 7	Section 8
C1*	C2*			C15 C16 C17	C18 C19 C20
270505			270506	270504	270507

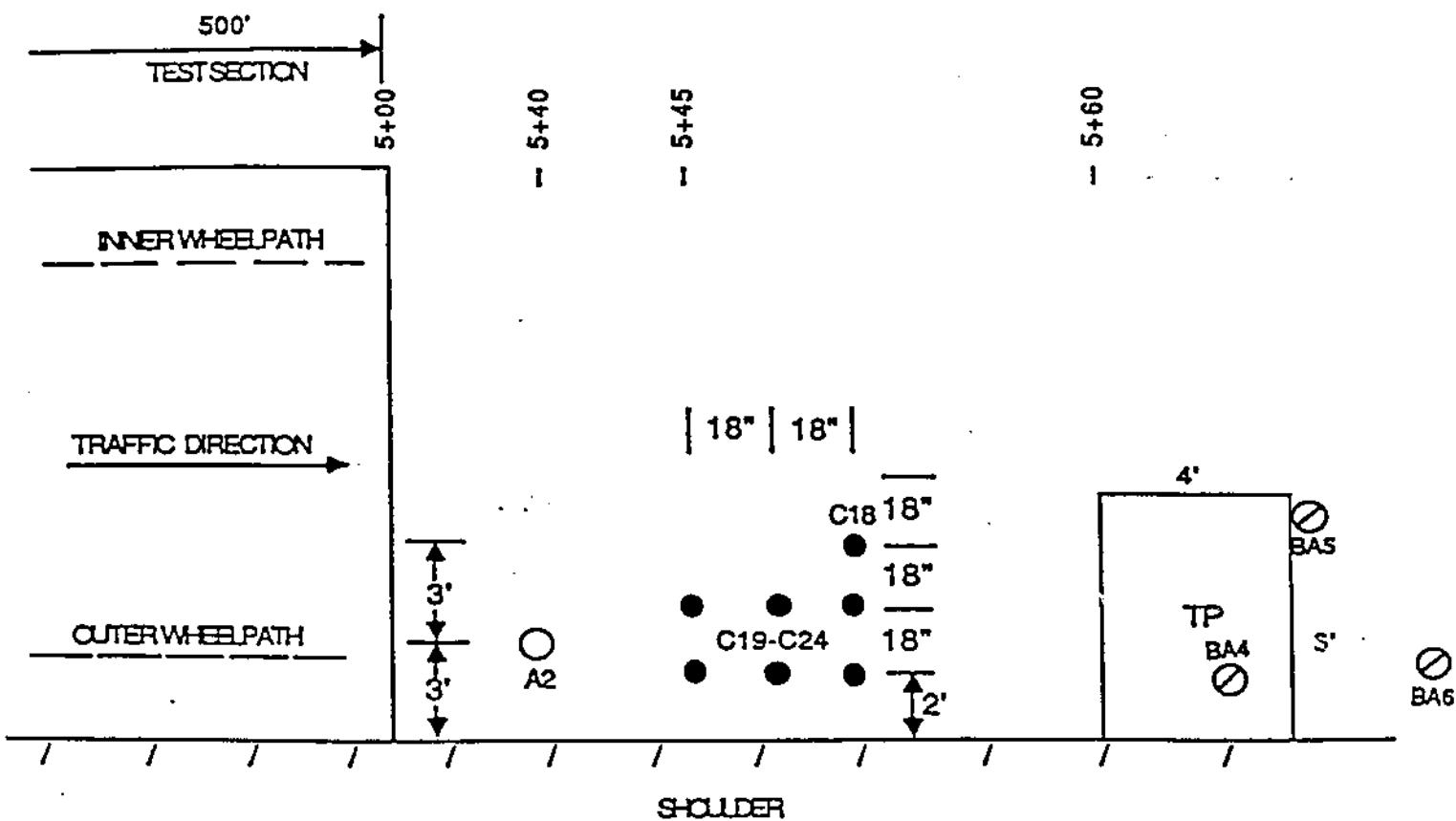
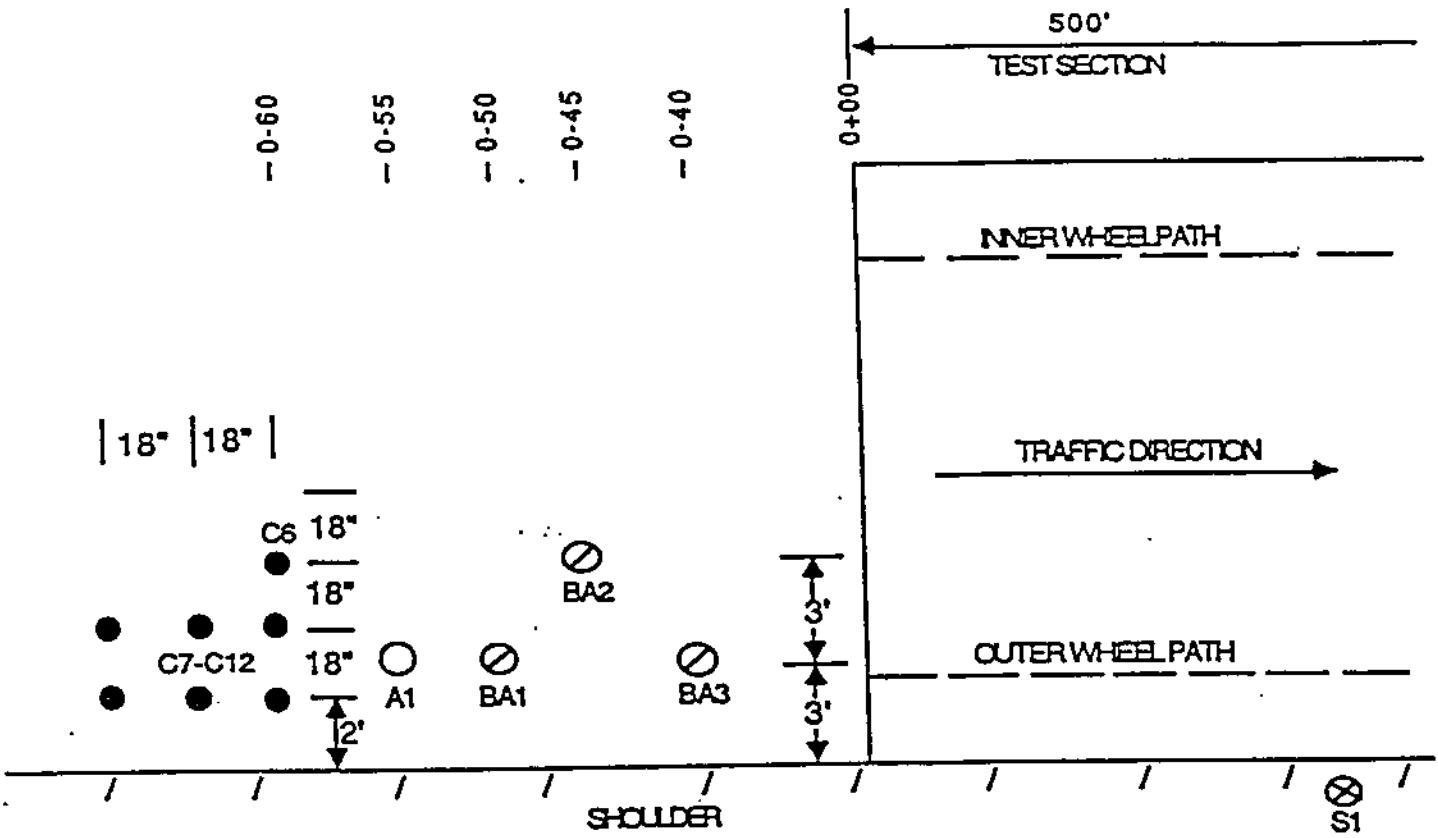
S2 •

		TRAFFIC FLOW			
		600	300	5393	Section 12
Section 9			Section 10	Section 11	
C1*	C2*			C31 C32 C33	270512
270511			270501	270510	

S3 •

S4 •

Figure 1 Proposed Sampling Plan



Sampling Point Locations After Test Section - Experiment 1
Asphalt Concrete over Granular Base



PAVEMENT
MANAGEMENT
SYSTEMS
PAVEMENT MANAGEMENT SYSTEMS

MEMORANDUM

TO: See Distribution

DATE: March 13, 1990

FROM: William A. Phang

FILE: 13.2.5

SUBJECT: Materials and Testing Plans for SPS 5 and SPS 6

In reviewing the Field Sampling and Laboratory Testing Memos for SPS 5 and SPS 6 dated January 30 and January 29/90 respectively, and the Tech Memos TM-PCS-1 and TM-PCS-2 dated February 23/90, "Rationale for Materials Testing Proposed for SPS-5 and SPS-6" respectively, the impression is very strong that these proposals are 'loaded' towards more testing than say a GPS because of the extent of the project and perhaps because of direct state funding.

So we may have a more austere perspective to consider at our March 23rd meeting. I have made some suggestions attached, which considerably reduce the size of the testing plan, without it is hoped compromising the quality of the data.

A point of note in both SPS 5 and SPS 6 layouts is the suggestion that the 500' test sections are spaced 300' apart. Is this a confirmed change from the 100' minimum transition?

Should we be considering FWD testing at this meeting? It has been put forward that this will be similar to FWD testing of GPS sections. It is my understanding that statistically speaking FWD tests from ten (10) test points should be capable of representing the characteristics of what is constructed as a uniform 500' test section. The ten points should conceptually be randomly selected. However because it is intended to follow the changes over time, measurement each time needs to be at the same points. This is more easily accomplished when the points are equally spaced, say at 100' intervals at lane center and in wheel track, for an SPS 5. Capturing the characteristics of an existing concrete pavement for an SPS 6 is more complex, but can be done by testing a small number of consecutive slabs, say five (5) to ten (10). Because of the overlays, it would be useful to test more of the joints than slab center and edges. A suggested pattern would be to test five points at slab center, five at pavement edge slab center, and 10 joints in the wheel track. This would allow testing of about 3 test sections per day, or completion of FWD testing at about the same time as drilling and sampling.

Distribution:

Amir Hanna
Adrian Pelzner
Guy Doré
Dennis Donnelly
Ed Harrigan
Jim Nichols
Brent Rauhut
Gene Skok
Bill Hadley
Gary Elkins

Sampling and Testing of SPS-5 Projects

Q. For the SPS-5 experiment, what needs to be known about the existing pavement for carrying out the analysis?

A 1. As a minimum, the asphalt pavement thickness, and the stiffness modulus of the asphalt concrete.

Based on the minimum, the following tests of the asphalt concrete are recommended:

- Measurement of pavement thickness from cores taken at each end of a test section
- Measurement of resilient modulus at each end of the project. Because at this time M_R testing is still mainly a research tool, stiffness values are often obtained by using the correlations developed by Van der Poel and Heukelom from common tests of mix air voids, asphalt content, volume concentration of the aggregate, and the penetration (at 77 ° F and 39.2 ° F) and softening point of the recovered asphalt cement.

The sampling requirements are:

1. 2-4" OD cores from each end of the test section. From these one can obtain thickness air voids, asphalt content and volume concentration of the aggregate.
2. 4-4" OD cores from each end of the project or near each end of the project (e.g., between sections 3 and 4, and 7 and 8). From these one obtain thicknesses and can do M_R testing as per the current SHRP protocol P07.
3. 1-12" core or block from say the middle of the project. The asphalt from Abson Recovery can be tested for penetration at 77 ° F and 39.2 ° F, and for softening point.

A 2. As a minimum, the layer thicknesses of the base and subbase, the physical make up of the materials, the relative condition/state, and the stiffness modulus. These properties are also needed for the subgrade.

The following tests and sampling requirements are:

- 3 or more 12" diameter access holes to the base, subbase, and subgrade, for natural moisture and auger samples of these materials. These are all taken in the same area as the 4" cores for M_R testing, say between sections 3 and 4. Split spoon or Shelby tube samples to 5' below top of subgrade would be taken through one of these access holes.

The auger samples of unbound material are combined in the laboratory and samples split off for testing. Tests are for particle size gradation and Atterberg Limits so that the materials may be classified, natural moisture contents, moisture density relationships, and resilient modulus.

A 6'x4' test pit from which a 12"x12" block of pavement is recovered, would permit in-place density determinations of base, subbase, and subgrade, would permit measurement of layer thickness, and would allow recovery of natural moisture samples and large samples of base, subbase, and subgrade. The test pit should be located near the second set of 4" M_R cores, i.e., between sections 7 and 8.

- A 3. For the overlays after construction, pavement thicknesses and stiffness modulus of the overlays are needed. Similar sampling as for the existing pavement i.e., 2-4" OD cores at each end of a section plus two sets each of eight 4" OD cores from two areas representing the virgin mix and the recycled mix.

A 4. Other Desirable Tests

1. Moisture susceptibility determinations of the overlay mixes, carried out during mix design of virgin and recycled mixes, could be helpful in future interpretation of pavement distress and durability performance. Moisture susceptibility determination of the existing pavement is of questionable value because it would have to be done on re-heated material, a process which does not yield similar results as for fresh mixes. Procedures for moisture susceptibility testing of cores have not yet been developed. Mix design will be done by the participating state, and any moisture susceptibility testing should be carried out at the same time. The state should be notified of any required testing, and given a SHRP protocol.
2. Creep compliance of the overlay mixes, both virgin and recycled, and of the existing pavement may be determined from cores. The results may be useful in future interpretation of rutting. C.P. Valkering, et al indicated that static creep testing results underestimate rut formation, so that a dynamic correction factor must be investigated ("Rutting Resistance of Asphalt Mixes Containing Non-Conventional and Polymer-Modified Binders", C.P. Valkering, D.J.L. Lancon, E. de Hilster, and D.A. Stoker, AAPT 1990). The SHRP project on bituminous mixes would be recommending meaningful tests on asphalt mixes. Because the current creep compliance test appears to be not inclusive, it is suggested that testing of this nature be postponed.
3. Permeability of unbound granular bases may be of some value in future performance interpretation. A SHRP protocol is needed. As permeability criteria are still being sought, it would be useful to have at least two permeability measurements in an SPS-5.

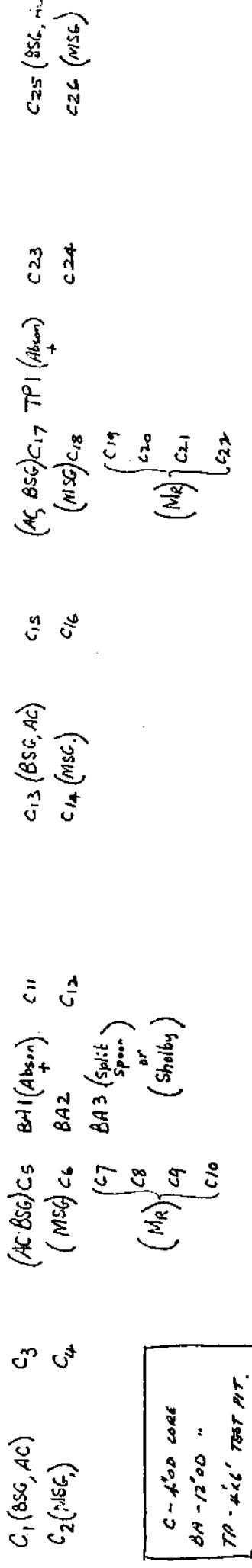
SAMPLING & TESTING SPS-5

SAMPLING PLAN

CONTROL.

(1)	(2) 2" Recycled	(3) 5" R	(4) 5" R	(5) 2" Virgin	(6) 2" V	(7) 5" R	(8) 5" R	(9) 2" R

PRE-CONSTRUCTION SAMPLING.



POST-CONSTRUCTION SAMPLING.

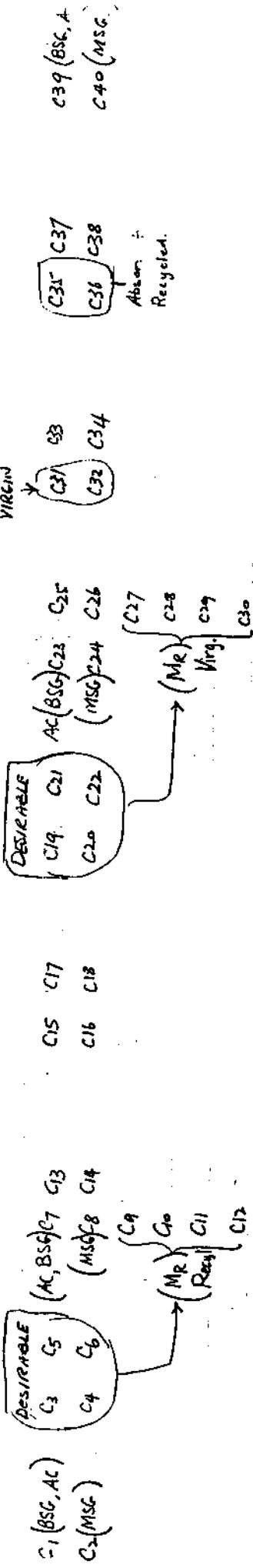


TABLE 1

SPS-5 LABORATORY TESTING PLANS (PRE-CONSTRUCTION)

Material Type and Properties	SHRP Designation / Procedure/ Method	Test Method	Material Source/ Test Locations			
			Tests/Samples per Layer	Test Site/ Layer		
PRE-CONSTRUCTION						
1. ASPHALT CONCRETE						
A. Asphaltic Concrete:						
Core Examination/Thickness	AC01	P01	SHRP-LTPP Method	All Cores (All Layers)		
Bulk Specific Gravity	AC02	P02	AASHTO T166-881	C1, C5, C9, C17, C25		
Maximum Specific Gravity	AC03	P03	AASHTO T209-82	C2, C6, C14, C18, C26		
Asphalt Content (Extraction)	AC04	P04	AASHTO T166-86	C1, C5, C13, C17, C25		
Moisture Susceptibility	AC05		AASHTO T283-871	1		
Creep Compliance	AC06		SHRP-LTPP Method	1		
Resilient Modulus	AC07	P07	ASTM D4123-82	C7, C8, C9, C19, C20, C21		
Tensile Strength	AC07	P07	ASTM D4123-82	C10, C22		
B. Extracted Aggregate:						
Bulk Specific Gravity:						
Coarse Aggregate	AG01		AASHTO T85-801			
Fine Aggregate	AG02	4	AASHTO T84-881			
Type and Classification:						
Coarse Aggregate	AG03		ASTM D2488-84			
Fine Aggregate	AG03		ASTM D2488-84			
Gradation of Aggregate	AG04	P1/4	AASHTO T50-871	C1, C5, C13, C17, C25		
C. Asphalt Cement:						
Asphalt Recovery	AE01		AASHTO T170-84			
Penetration at 77F & 39.2°F	AE02		AASHTO T49-871			
Ductility at 77F	AE03		AASHTO T51-871			
Specific Gravity (SGF)	AE04		AASHTO T220-871			
Ring and Ball Softening Pt.	AE05		AASHTO T53-871			
Viscosity at 77F	AE06		AASHTO T202-84			
Viscosity at 140F	AE06		AASHTO T202-84			

1/1

TABLE 1 cont'd

SPS-5 LABORATORY TESTING PLANS (PRE-CONSTRUCTION)

Material Type and Properties	SHRP Designation/ further	Size/ Method	Test Method	Tests/Samples per-layer	Material Source/ Test Locations
III. BOUND (TREATED) BASE AND SUBBASE					
Type and Classification of Material and Treatment	TB01	P31	ASTM D2488-84 ASTM D2166-66(TP1)	4	
Compressive Strength	TB02	P32	ASTM C39-83	4	
Dynamic Modulus (77F)	TB03	P33	ASTM D3697-78(85)	4	
III. UNBOUND GRANULAR BASE AND SUBBASE					
Particle Size Analysis Sieve Analysis (washed)	UG01	P41	AASHTO T27-881	4.2	BA1 BA2 BA3 TP1
Hydrometer to 0.001mm	UG02	P41	AASHTO T11-85	4.2	BA1 BA2 BA3 TP1
? — Atterberg Limits	UG03	P43	AASHTO T88-86	4	
Moisture-Density Relations	UG04	P44	AASHTO T89-871	4	
? — Laboratory CBR	UG05	P44	AASHTO T189-86	4.1	BA1 BA2 BA3 TP1
Resilient Modulus	UG07	P46	AASHTO T274-82	4.1	BA1 BA2 BA3 TP1
Classification	UG08	P47	ASTM D2488-84	4.2	BA1 BA2 BA3 TP1
? — Permeability	UG09	?	ASTM D2434-68(TP1)	4.2	BA1 BA2 BA3 TP1
Natural Moisture Content	UG10	P49	AASHTO T265-86	4.4	BA1 BA2 BA3 TP1
IV. SUBGRADE					
Soil Analysis	SS01	P51	AASHTO T27-881	4.2	BA1 BA2 BA3 TP1
Hydrometer to 0.001mm	SS02	P42	AASHTO T88-86	4.2	BA1 BA2 BA3 TP1
Atterberg Limits	SS03	P43	AASHTO T89-871	4.2	BA1 BA2 BA3 TP1
Classification	SS04	P52	AASHTO M145-82 ASTM D2488-84	28.2	BA1 BA2 BA3 TP1
Moisture-Density Relations	SS05	P55	AASHTO T99-86/ AASHTO T180-86	4.1	BA1 BA2 BA3 TP1
? — Laboratory CBR	SS06	P46	AASHTO T193-81	4	
Resilient Modulus	SS07	P46	AASHTO T274-82	4	
? — Unit Weight	SS08	P49	SHRP-TTP Method	4	
Natural Moisture Content	SS09	P49	AASHTO T265-86	4	
Unconfined Compressive Strength	SS10	P49	AASHTO T208-70(S4)	4.1	BA1 BA2 BA3 TP1

X

TABLE 2

SPS-5 LABORATORY TESTING PLANS (POST-CONSTRUCTION)

Material Type and Properties	SHRP Designation	Test Method	Tests/Samples per Layer	Material Source/ Test Locations				
				Testable	Testable			
POST-CONSTRUCTION								
I. ASPHALT CONCRETE								
A. Asphaltic Concrete:								
Core Examination/Thickness	AC01	SHRP-LTPP Method AASHTO T166-87I	25-40	All Cores C1 C7 C23 C37				
Bulk Specific Gravity	AC02		4	C2 C8 C24 C40				
Maximum Specific Gravity	AC03	AASHTO T209-82	4	C1 C7 C23 C37				
Asphalt Content (Extraction)	AC04	AASHTO T164-86	1		DESTRUCTIVE			
Moisture Susceptibility	AC05	AASHTO T203-87I	1					
Creep Compliance	AC06	SHRP-LTPP Method	4	C4 C10 C11 C14 C15 C16				
Resilient Modulus	AC07	ASTM D4123-82	4	C27 C28 C29				
Tensile Strength	AC07	ASTM D4123-82	4	C30	C19			
B. Extracted Aggregate:								
Bulk Specific Gravity:								
Coarse Aggregate	AG01	AASHTO T85-88I	4					
Fine Aggregate	AG02	AASHTO T84-88I	4					
Type and Classification:								
Coarse Aggregate	AG03	ASTM D2429-84	4					
Fine Aggregate	AG03	ASTM D2429-84	4					
Gradation of Aggregate	AG04	AASHTO T10-87I	4	C1 C7 C23 C39				
C. Asphalt Cement:								
Asphalt Recovery	AE01	AASHTO T170-84	4	VIRGIN C31 C32	RECYCLED C35 C36			
Penetration at 77°F # 37.2°F	AE02	AASHTO T49-87I	2	C31 C32	C35 C36			
Ductility at 77°F	AE03	AASHTO T51-87I	4					
Specific Gravity (60°F)	AE04	AASHTO T226-87I	6					
Ring and Ball Softening Pt.	AE05	AASHTO T33-87I	2					
Viscosity at 77°F	AE06	AASHTO T202-84	4					
Viscosity at 160°F	AE06	AASHTO T202-84	4					

Sampling and Testing of SPS-6 Projects

Q. For the SPS-6 experiment, what needs to be known about the existing pavement for carrying out the analysis?

A 1. Asphalt Overlays on concrete pavement develop problems at joints and cracks in the underlying concrete. It is important to know where these cracks and joints are located. Pavement thicknesses are important, as are stiffnesses measured from cores or deduced from deflection measurements. The presence of voids under a joint is useful information. A knowledge of the coefficient of expansion of the concrete is useful.

A minimum number of results to characterize material properties of the concrete cores is perhaps five. SHRP protocol P61 (Compressive Strength) and P64 (Static Modulus of Electricity) require testing of 4" OD cores, while P62 (Splitting Tensile Strength) require testing of 6" OD cores. It is proposed that five sets 4" and 6" cores for these tests be taken (at every other transitions), with two additional 4" cores at the ends of the project. These additional cores will be used if M_R testing of asphalt treated subbase is needed.

To obtain bulk samples of subbase and subgrade, access is proposed through 3-12" core holes at each end of the project. Tests for coefficient of expansion may be carried out on blocks sawn from the 12" OD core, or on the additional 4" cores. Voids under the pavement may be inferred from FWD tests at joints or from evidence of pumping or faulting.

A 2. As in concrete GPS test sections, a test pit will not be required, although there will be then no opportunity to measure layer thickness or the in-place density of unbound subbase or the subgrade. However if full depth patches are part of the intensive CPR, with cooperation of the contractor, it may still be possible to carry out such in-situ measurement.

There seems to be no need for CBR testing, as resilient modulus tests are to be performed on the subgrade materials. Permeability measurement of unbound granular subbase may prove valuable.

A 3. (It is proposed in post-construction sampling to take 10-4" OD cores and 20-6" OD cores, and to use the 6" OD cores for tests for Bulk S.G., Max. S.G., and asphalt content. Since all previous tests by these procedures use 4" OD cores, the reason for changing to 6" OD cores is unclear).

SAMPLING & TESTING SPS-6.

SAMPLING PLAN

SECT. NO.	① CONTROL	②	⑤	⑥	③	④	⑦	⑧
SAMPLING AREA → 1	2 3	intensive CPB	min. restoration	4 9	4" AC min. restoration	4" AC Seawall/ min. restoration	4" AC break / seal	8" AC break / seal

API 2

PRE- CONSTRUCTION.

SH - C1 BA1 - C4.	SH - C5	Craft
SH - C2 BA2	Craft	SH - C4
ST - C3 BA3	ST - C6	Shp. C15
C4	ST - C7	ST - C15
C5	C6	ST - C16
C7	C7	BA5
C8	C8	BA6
C9	C9	BA7
C10	C10	BA8
C11	C11	BA9
C12	C12	BA10
C13	C13	BA11
C14	C14	BA12
C15	C15	BA13
C16	C16	BA14
C17	C17	BA15
C18	C18	BA16
C19	C19	BA17
C20	C20	BA18

C - 4" OD core
④ - 6" OD core.
BA - 12" OD core

POST- CONSTRUCTION

AC BSC C1	SH C3 CS BSC AC	SH C7 CA BSC AC	SH C11 C13 BSC AC	SH C15 C17 BSC AC	SH C19 BSC
MSC C2	MSC C6 MSC	MSC C8 C10 MSC	MSC C12 C14 MSC	MSC C16 C18 MSC	MSC C20 MSC

TABLE I. SPS-6 LABORATORY TESTING PLANS (PRE-CONSTRUCTION)

*SHPD
Protocol*

Material Type and Properties	SDTF Test Designation	Test Method	Test/Layer	Material Source/ Test Location
I. PORTLAND CEMENT CONCRETE				
Compressive Strength	PC01	ASTM T22-88T	26.5	C2 C6 C9 C12 C15.
Splitting Tensile Strength	PC02	ASTM T198-83	6	C3 C7 E10 C13 C16
PCC Coefficient of Thermal Expansion	PC03	ASTM C160-87	4.5	C1 C5 C8 C11 C14
Static Modulus of Elasticity	PC04	SDTF-LTTF Matched	26	
PCC Unit Weight / Thickness	PC05	SDTF-LTTF Matched	4.5-17	All Cores
Core Examination / Thickness	PC06			
II. SOILED (TREATED) BASE AND SUBGRADE				
Type and Classification of Material and Type of Treatment	T001	ASTM D2166-66(79)	3	C2 C6 C9
Compressive Strength	T002	ASTM D2166-66(79) AND ASTM C19-83	3.2	C1 C4 C5 C8 C11 C14
Dynamic Modulus (3 cores/ test)	T003	ASTM D3497-78(85)	4	
III. INGROWN GRANULAR BASE AND SUBGRADE				
Particle Size Analysis	UC01	ASTM T27-88T	4.2	B41 B42 B43 B44 B45 B46
Sieve Analysis (washed)	UC02	ASTM T11-85	4.2	" "
Axes/Borders Limits	UC04	ASTM T89-87	4.2	" "
Moisture-Density Relations	UC03	ASTM T180-86	4	" "
Lab. CBR (at In-situ density and moisture; sieved and unsoaked)	UC04	ASTM T193-81	4	
Resilient Modulus (at in-situ density and moisture)	UC07	ASTM T274-82	4.2	B41 B42 B43 B44 B45 B46
Classification	UC08	ASTM D2488-84	4.2	" "
Permeability	UC09	ASTM D2434-68(79)	4.2	" "
Natural Moisture Content	UC10	ASTM T265-86	4.2	B41 B42 B43 B44 B45 B46

of dia. core

TABLE 1 (CONT'D) SFS-6 LABORATORY TESTING PLANS (PRE-CONSTRUCTION)

Material Type and Properties	SFSF Test Designations	Tests Method	Tests/Layer	Material Source/ Test Location
III. SUBGRADE				
Sieve Analysis	SSD1	AASHTO T27-88T	4.2	BAT BAT 6A3 BAT 6A6
Hydrometer to 0.001mm	SSD2	AASHTO T88-86	4.2	" "
Atterberg Limits	SSD3	AASHTO T89-87T/ AASHTO T90-87I	4.2	" "
Classification and Type of Subgrade	SSD4	AASHTO M145-82/ ASTM D2488-84	4.2	" "
Moisture-Density Relations	SSD5	AASHTO T99-86/ AASHTO T180-86	4.2	" "
Lab. CBR (at in-situ density and moisture; sealed and unsealed)	SSD6	AASHTO T193-81	4	
Pavement Modulus (at in-situ density and moisture)	SSD7	AASHTO T274-82	4.2	BAT BAT 6A3 BAT 6A6
Unit Weight	SSD8	SHP-LTTT Method	4.6	BAT BAT 6A3 BAT 6A6
Natural Moisture Content	SSD9	AASHTO T263-86	4.6	BAT BAT 6A3 BAT 6A6
Unconfined Comp. Strength	SSD10	AASHTO T208-70(B4	4.6	SHELLY TUBES

TABLE 2. SPS-4 LABORATORY TESTING PLANS (POST-CONSTRUCTION)

Material Type and Properties	SPS Test Designation	Test Method	Tests/Lot No.	Material Source/ Test Location
I. ASPHALT CONCRETE				
Core Examination/Thickness	P01	SHPD-LTPP Methoded	20/20	PIER CORES
Bulk Specific Gravity	P02	AASHTO T166-881	C1/C2	C14
Maximum Specific Gravity	P03	AASHTO T209-82	C3	C5
Asphalt Content	P04	AASHTO T166-86	C4	C20
Moisture Susceptibility	P05	AASHTO T263-871	C5	C18
Creep Compliance	P06	SHPD-LTPP Methoded	1	C14
Resilient Modulus	P07	ASTM D4123-82	C1/C2	C14
Tensile Strength	P07	ASTM D4123-82	C3/C4/C7/C8	C14
			C1/C2	C14

—Extrn. = Extraneous
Extm. = External

SPS 5
14 MILES WEST OF BEMIDJI MIN
WEST BOUND TH 2

According To March 29, 1990
Sampling plan.

TRAFFIC FLOW

SECTION	TP 1	SECTION	TP 2	SECTION	TP 3	SECTION	TP 4	SECTION	TP 5	SECTION	TP 6	SECTION	TP 7	SECTION	TP 8	SECTION	TP 9	SECTION	TP 10	SECTION	TP 11	SECTION	TP 12	SECTION	TP 13	SECTION	TP 14	SECTION
C-1	TP 1 C-4 C-5 A-1 C-3 C-4	SECTION 2	TP 1 C-5 C-6	SECTION 3	TP 1 C-6	SECTION 4	TP 1 C-7 C-8 (6")	SECTION 5	TP 1 C-9	SECTION 6	TP 1 C-10 C-11 C-12	SECTION 7	TP 1 C-13 C-14	SECTION 8	TP 1 C-15 C-16 C-17 C-18	SECTION 9	TP 1 C-19 C-20 C-21	SECTION 10	TP 1 C-22 C-23 C-24	SECTION 11	TP 1 C-25 C-26 C-27	SECTION 12	TP 1 C-28 C-29 C-30	SECTION 13	TP 1 C-31 C-32 C-33	SECTION 14	TP 1 C-34	
C-2	270503	270508	270502	270509	270507	270507	270507	270509	270509	270506	270506	270506	270506	270506	270506	270506	270506	270506	270506	270506	270506	270506	270506	270506	270506	270506		
S-1	307425	31275	316400	321400	323350	328450	333350	338450	343350	348450	353350	358450	363350	368450	373350	378450	383350	388450	393350	398450	403350	408450	413350	418450	423350	428450		
	307425	31275	316400	321400	323350	328450	333350	338450	343350	348450	353350	358450	363350	368450	373350	378450	383350	388450	393350	398450	403350	408450	413350	418450	423350	428450		
	307425	31275	316400	321400	323350	328450	333350	338450	343350	348450	353350	358450	363350	368450	373350	378450	383350	388450	393350	398450	403350	408450	413350	418450	423350	428450		
	307425	31275	316400	321400	323350	328450	333350	338450	343350	348450	353350	358450	363350	368450	373350	378450	383350	388450	393350	398450	403350	408450	413350	418450	423350	428450		
	307425	31275	316400	321400	323350	328450	333350	338450	343350	348450	353350	358450	363350	368450	373350	378450	383350	388450	393350	398450	403350	408450	413350	418450	423350	428450		
	307425	31275	316400	321400	323350	328450	333350	338450	343350	348450	353350	358450	363350	368450	373350	378450	383350	388450	393350	398450	403350	408450	413350	418450	423350	428450		

SPS-5 PROJECT - 14 MILES WEST OF BEMIDJI, MN WESTBOUND TH 2

FROM BRAUN PUMT TECH INC

TRAFFIC FLOW

Final

Section 1	50'	325'	250'	150'	Section 5
Section 2					Section 4
270503	C3* C4* A1	270508	[C5 C6]	270509	270509
C1* C2*					*C10 *C11 *C12

51 •

Section 5	700'	450'	350'	8400	Section 9
Section 6					Section 8
270505	[C13 C14 C15] [C16 C17]	270506	*C15 *C16 *C17	270507	270507
C4*					*C18 *C19 *C20 A3-C23

S2 •

Section 9	600'	300'	5393	Section 12
Section 10				Section 11
270511	A4-C24 [C25]	270501	*C21 *C30	270510
C4*				270512
C25*				*C25 *C26

C = 40

NO. 5

PAGE 15

(FRI) 06.01.90 13:28

According To No. h 29,1490
Sampling plan.

Section	50'	Section	325'	Section	250'	Section	150'	SECTION
1	270503	2	270508	3	270502	4	270507 <th>5</th>	5

Section	700'	Section	450'	Section	350'	Section	2400'	SECTION
5	270505	6	270506	7	270504	8	270507	9

Section	600'	SECTION	300'	SECTION	5393	SECTION	12
9	270511	10	270501	11	270510	12	270510

TABLE 1

SPS-5 LABORATORY TESTING PLANS (PRE-CONSTRUCTION)

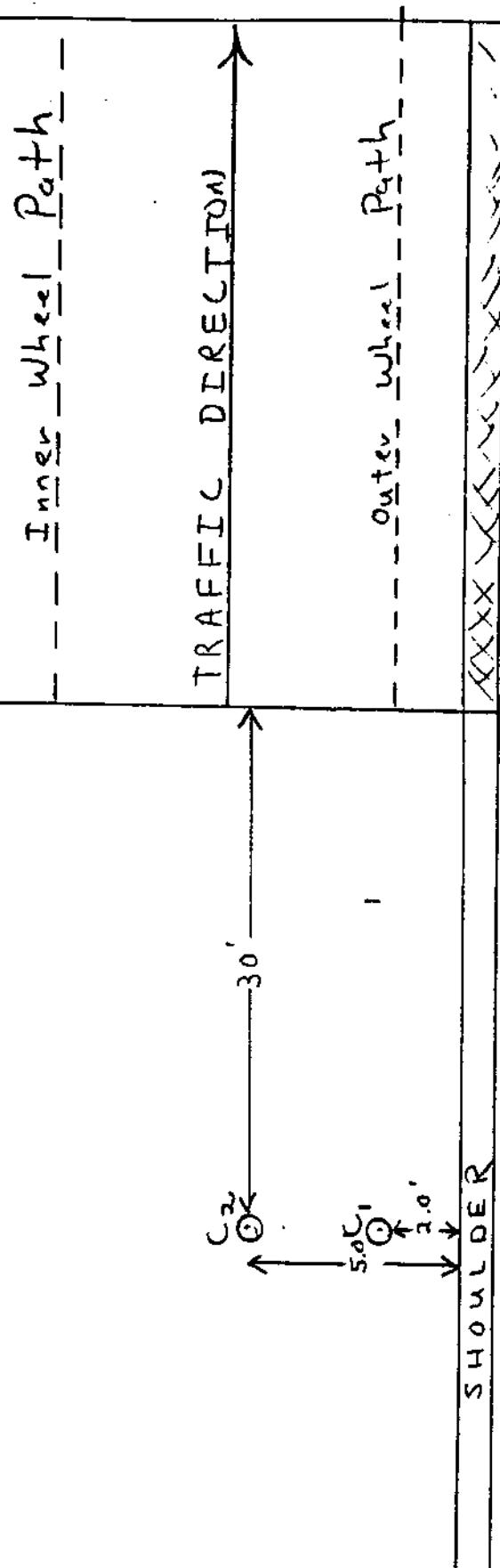
Material Type and Properties	SHRP Designation	GPS Protocol	Tests/Samples per Layer	Material Source/ Test Locations
PRE-CONSTRUCTION				
I. ASPHALT CONCRETE				
A. ASPHALTIC CONCRETE:				
Core Examination/Thickness	AC01	P01	26	ALL CORES
Bulk Specific Gravity	AC02	P02	9	C2 [C3, C4] C13 [C14, C15] C22 [C23, C24] ¹
Maximum Specific Gravity	AC03	P03	3	[BA1-3] [TP] [BA4-6]
Asphalt Content (Extraction)	AC04	P04	3	[BA1-3] [TP] [BA4-6]
Moisture Susceptibility	NOTE 1	3	A1 A2 A3
Creep Compliance	NOTE 2	3	C5 C9 C20
Resilient Modulus	AC07	P07	3	[C3, C4] [C14, C15] [C23, C24]
Tensile Strength	AC07	P07	3	C2 C13 C22
B. EXTRACTED AGGREGATE:				
Type and Classification:	AG03	NOTE 3	3	[BA1-3] [TP] [BA4-6]
Coarse Aggregate	AG03	3	[BA1-3] [TP] [BA4-6]
Roundness Index of Coarse Aggregate	AG03	3	[BA1-3] [TP] [BA4-6]
Fine Aggregate	3	[BA1-3] [TP] [BA4-6]
NAA Test for Fine Aggregate Particle Shape	AG05	P14A NOTE 3	3	[BA1-3] [TP] [BA4-6]
Aggregate Particle Shape	AG04	P14	3	[BA1-3] [TP] [BA4-6]
Gradation of Aggregate	3	[BA1-3] [TP] [BA4-6]
C. ASPHALT CEMENT:				
Abson Recovery	AE01	3	[BA1-3] [TP] [BA4-6]
Penetration at 50F, 77F, 90F	AE02	3	[BA1-3] [TP] [BA4-6]
Specific Gravity (60F)	AE04	3	[BA1-3] [TP] [BA4-6]
Viscosity at 77F	ASTM D3205-B6	3	[BA1-3] [TP] [BA4-6]
Viscosity at 140F, 275F	AE06	3	[BA1-3] [TP] [BA4-6]

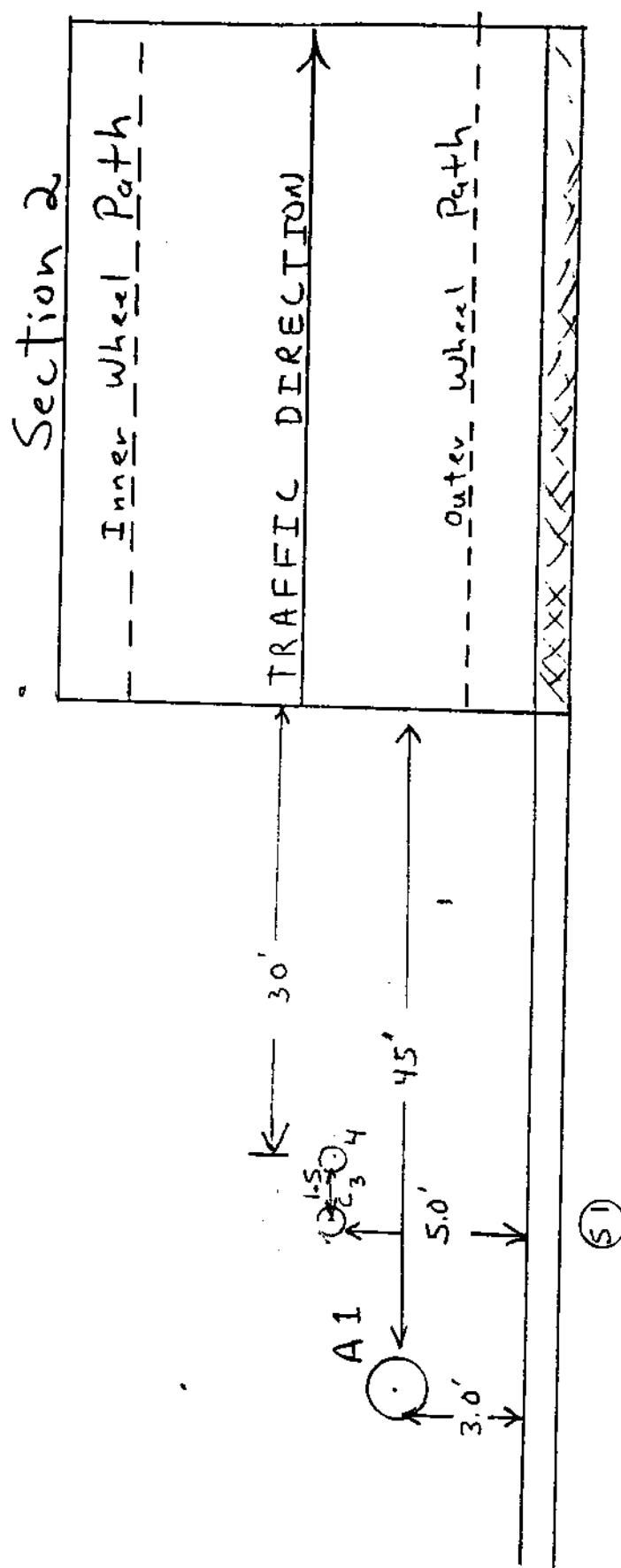
- NOTES: 1 Moisture susceptibility will be assessed by visual means on cores split in the field. If traveled around using or stored in the field, cores will be integrated.
- 2 Creep compliance will be performed when suitable procedures are developed -- cores will be stored.
- 3 National Aggregate Association will perform tests at no cost to the State.
- 4 Cores within brackets are from the same sampling location.

TABLE 1 cont'd

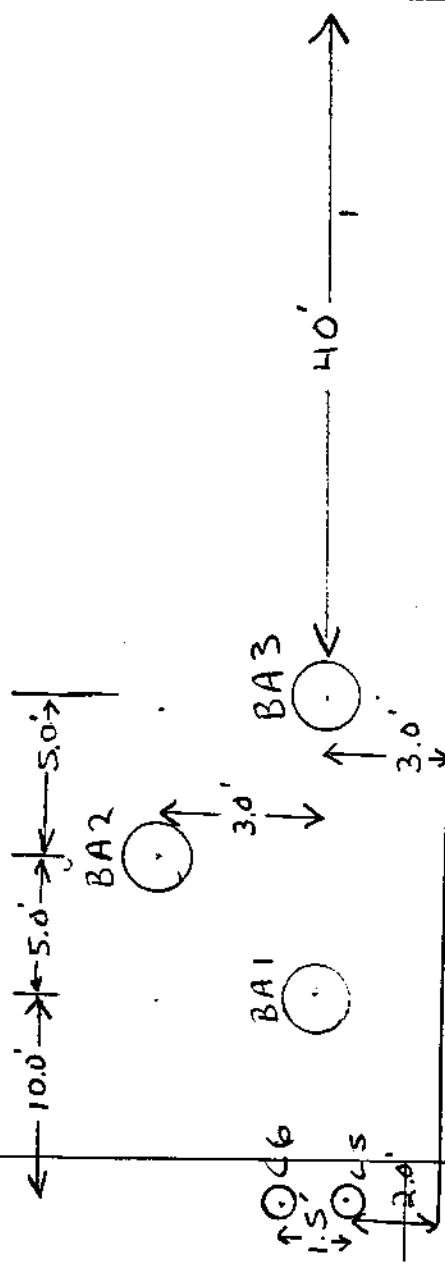
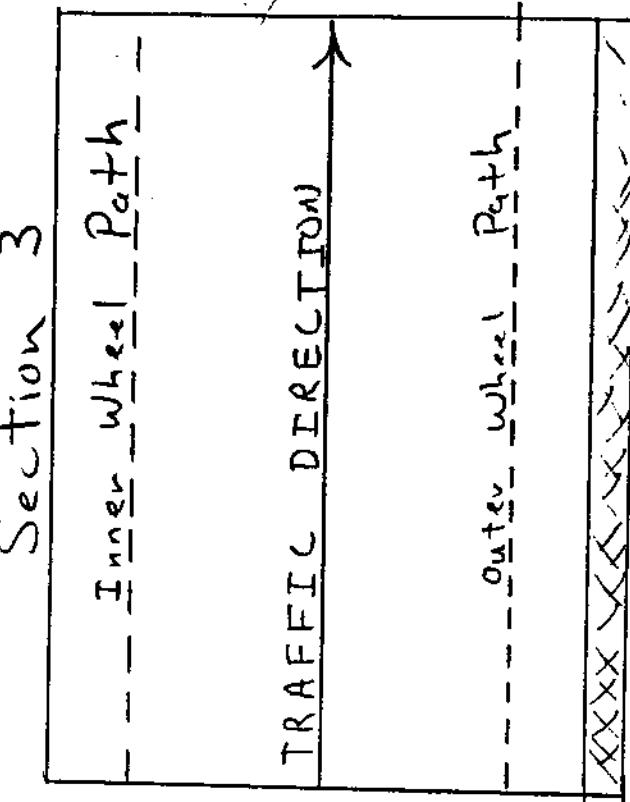
Material Type and Properties	SHRP Designation	SHRP GPS Protocol	Tests/Samples per Layer	Material Source/ Test Locations
II. BOUND (TREATED) BASE AND SUBBASE				
Type and Classification of Material and Treatment	TB01	P31	3	C3, C4 C14, C15 C23, C24
Pozzolanic/Cementitious: Compressive Strength	TB02	P32	3	C3, C4 C14, C15 C23, C24
Asphalt treated: Dynamic Modulus (77F)	TB03	P33	3	C3, C4 C14, C15 C23, C24
HMAC: Resilient Modulus	AC07	P07	3	C3, C4 C14, C15 C23, C24
III. UNBOUND GRANULAR BASE AND SUBBASE				
Particle Size Analysis	UC01	P41	3	[BA1-3] [TP] [BA4-6]
Sieve Analysis (washed)	UC02	P41	3	[BA1-3] [TP] [BA4-6]
Atterberg Limits	UG04	P43	3	[BA1-3] [TP] [BA4-6]
Moisture-Density Relations	UG05	P44	3	[BA1-3] [TP] [BA4-6]
Resilient Modulus	UG07	P46	3	[BA1-3] [TP] [BA4-6]
Classification	UG08	P47	3	[BA1-3] [TP] [BA4-6]
Permeability	UG09	P48	3	[BA1-3] [TP] [BA4-6]
Natural Moisture Content	UG10	P49	3	[BA1-3] [TP] [BA4-6]
IV. SUBGRADE				
Steve Analysis	SS01	P51	3	[BA1-3] [TP] [BA4-6]
Hydrometer to 0.001mm	SS02	P42	3	[BA1-3] [TP] [BA4-6]
Atterberg Limits	SS03	P43	3	[BA1-3] [TP] [BA4-6]
Classification	SS04	P52	3	[BA1-3] [TP] [BA4-6] A1 A2 A3
Moisture-Density Relations	SS05	P55	3	[BA1-3] [TP] [BA4-6]
Resilient Modulus	SS07	P46	3	[BA1-3] [TP] [BA4-6]
Unit Weight	SS08	P48	3	[BA1-3] [TP] [BA4-6] A1 A2 A3 —
Natural Moisture Content	SS09	P49	3	[BA1-3] [TP] [BA4-6]
Depth to Rigid Layer			3	S1 S2 S3

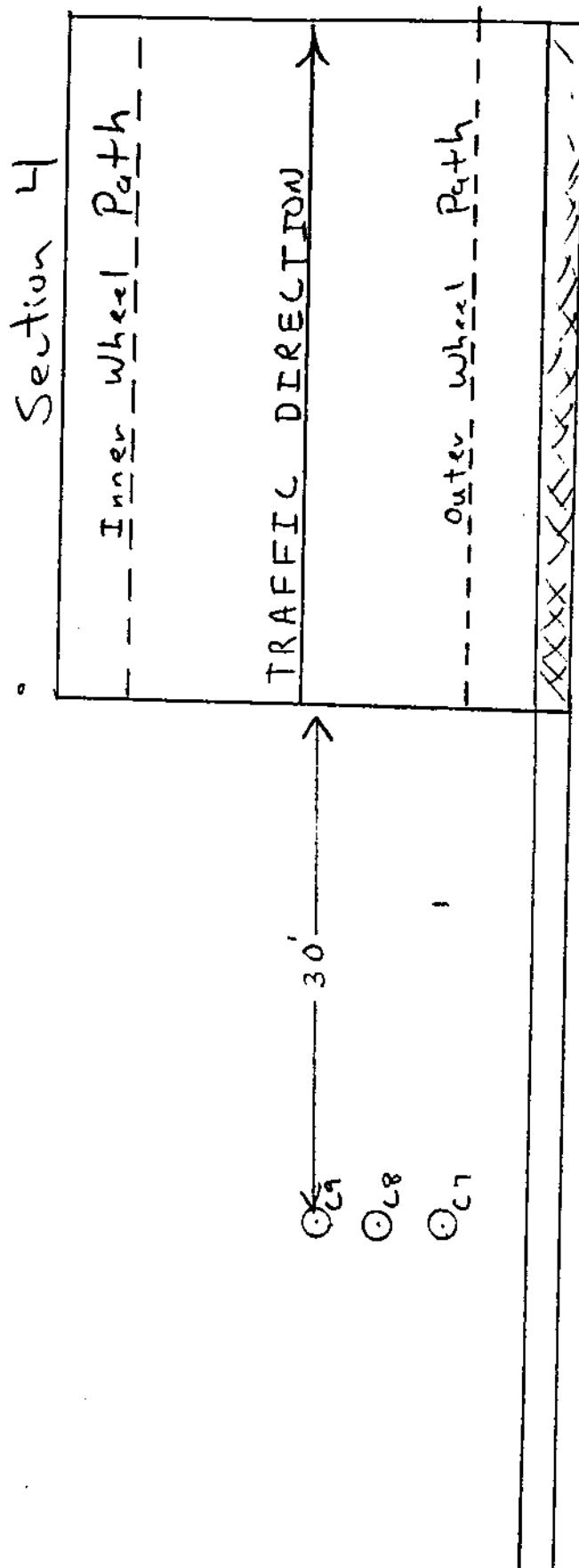
Section 1

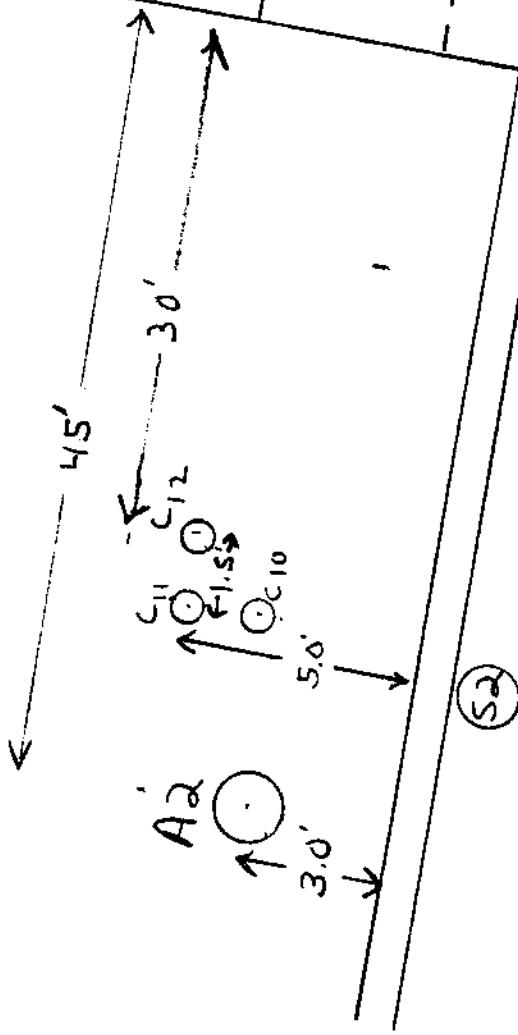
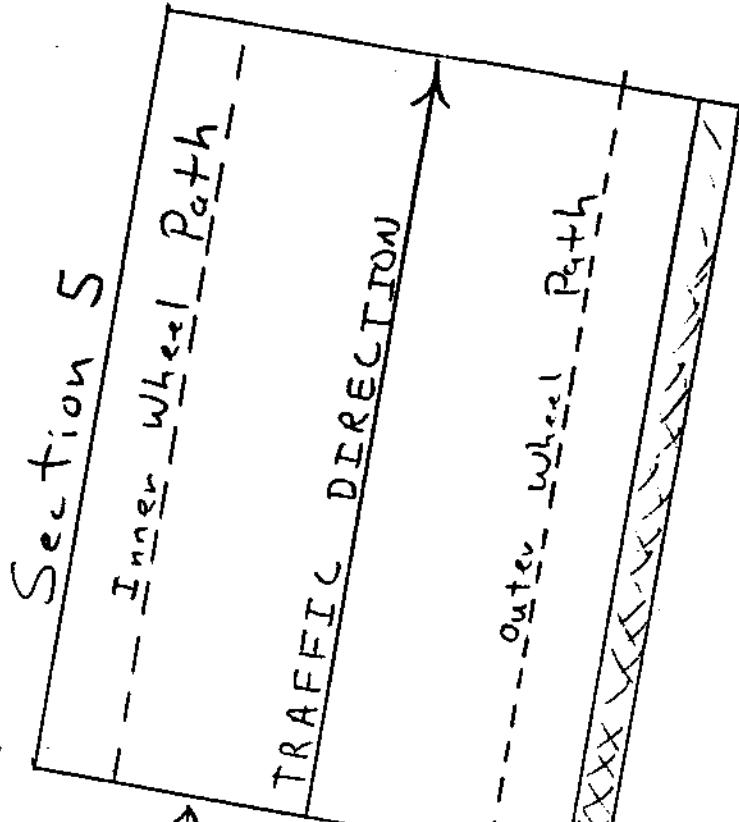


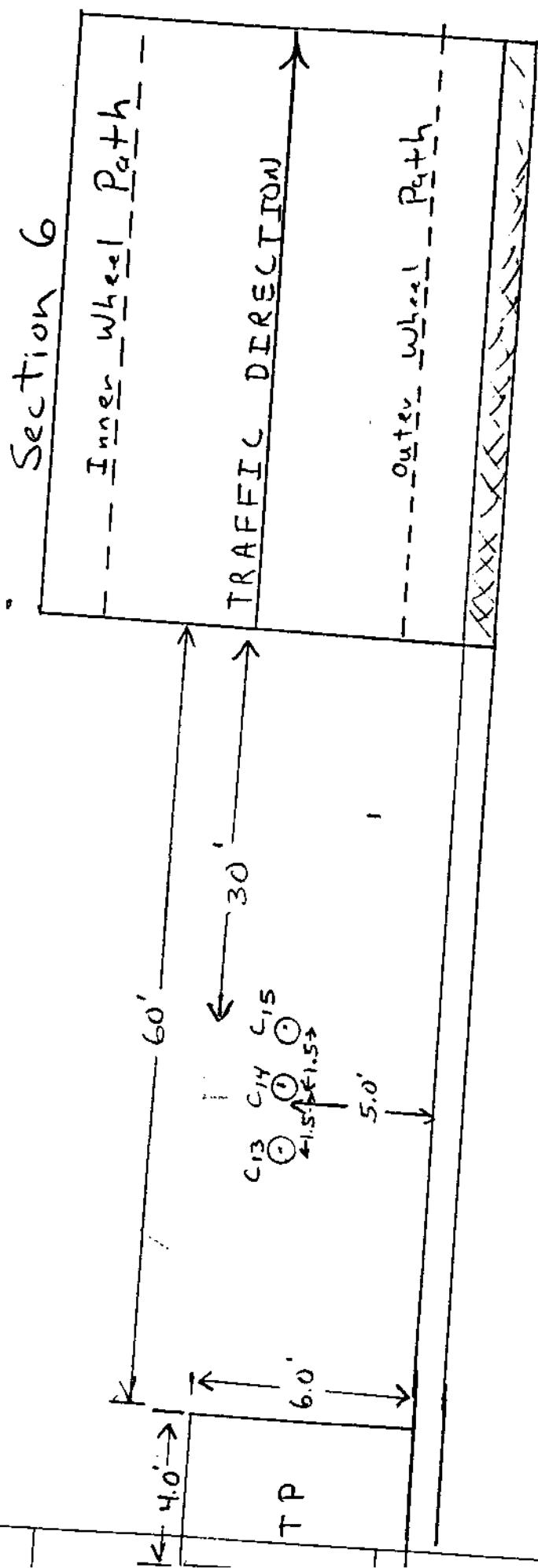


Section 3

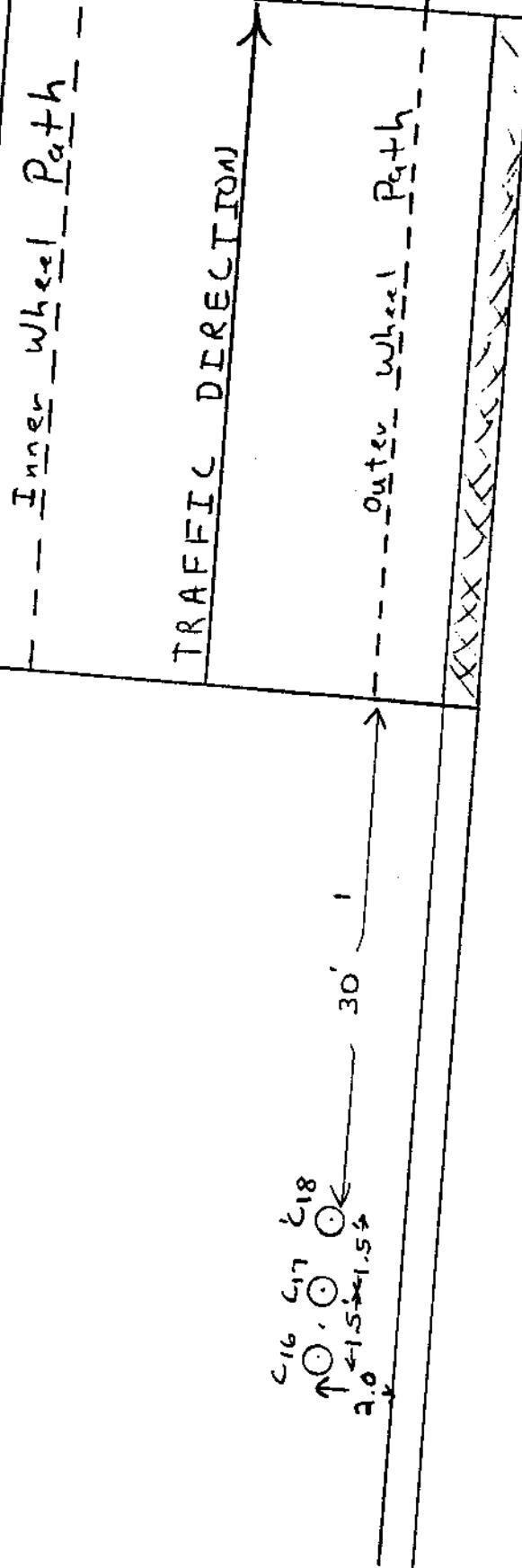


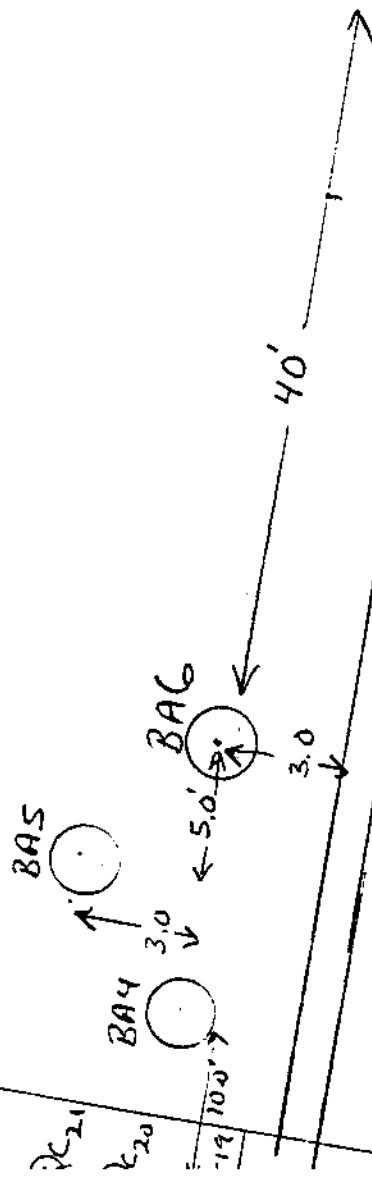
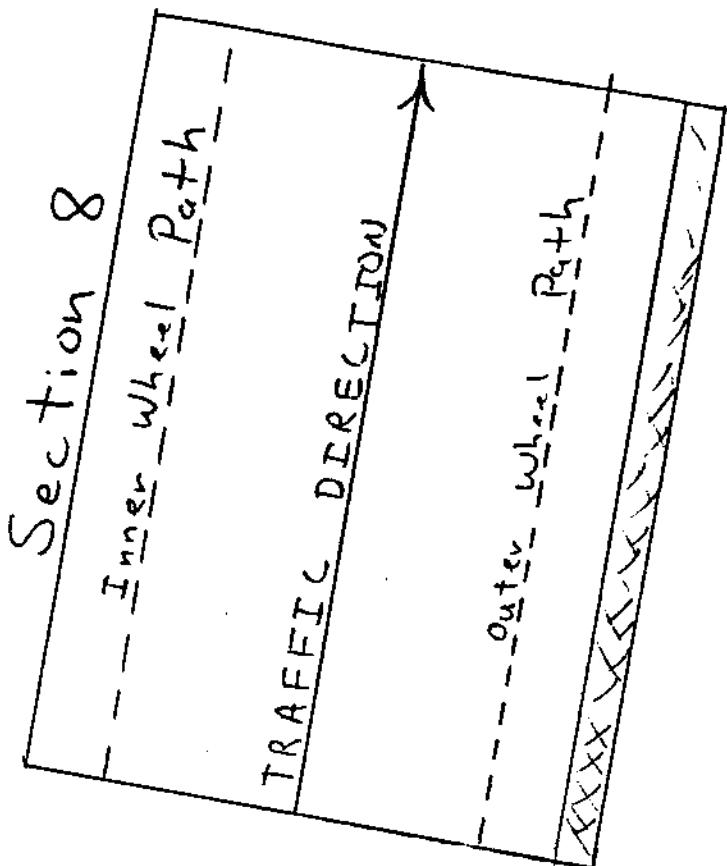




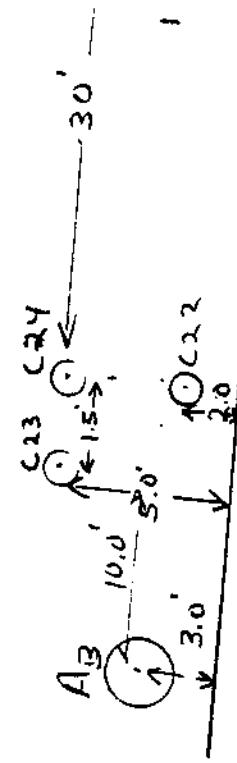
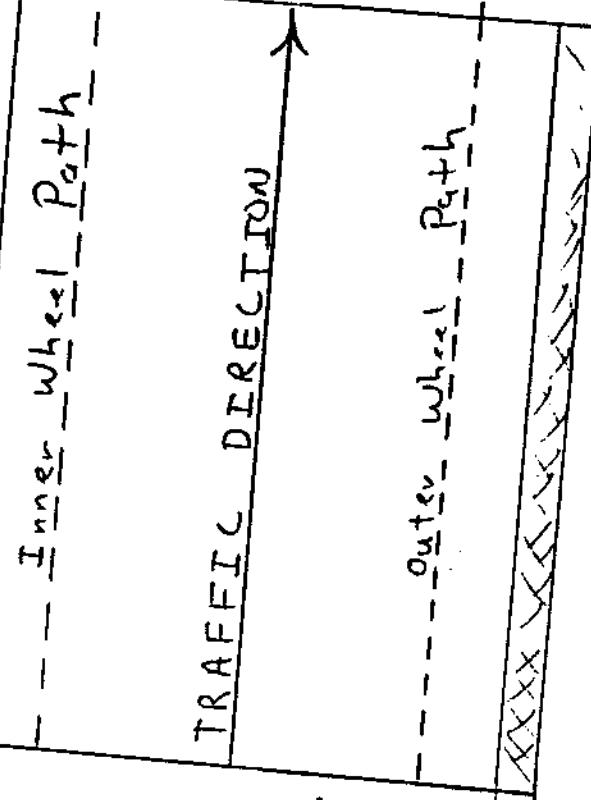


Section 7





Section 9



Section 9

Inner - Wheel Path

TRAFFIC DIRECTION

Outer - Wheel Path



T.P. Removal of Pavement layers
Collection of Slabs; Nuclear Density and
Moisture Measurements on Unstabilized layers
and Subgrade.

TP1	OLD		New	
	Extracted Ag Spec Gr. Coarse, fine	4,4	Extracted Ag Type And Class Coarse, Fine Roundness index of Coarse Aggregate particle shape Gradations	3,3 3,3 3,3 3,3
Type Agg and glass for Coarse and fine Gradations	4,4			
AC	4			
Absor recovery, Pen. 77 Ductility, 77 Spec. G. Rings and Ball Sutpt., Vis 77, 140	4,4 4,4 4,4,4		AC Max Spec G. - AC coded Absor recovery, (pen 50,7790) Spec G, (60°F), Vis 77, 140 225	3,3 3,3 3,3,3
<u>BASE AND SUBBASE</u>			<u>Base and Subbase</u>	
Particle size, Sieve Anal, Hydro, Atterberg, Moisture-Density, Lab CBR, Resist Mod, Classif, Permeability, Nat. M.C.	4,4,4 4,4 4,4 4,4,4		Particle size, sieve Atterberg, Moisture Density, Resist Mod, Class Permeability, Nat. M.C.	3,3 3,3 3,3 3,3
<u>Subgrade</u>			<u>Subgrade</u>	
Sieve, Hydrometer, Atterberg Classification, Moisture Density, Lab CBR, Resist Mod, Unit wt Nat. M.C., Unclassified Comp. St.	4,4 4,4 4,4 4,4		Sieve, Hydrom, Atterberg Classification, Moisture Density Resist Mod, Unit wt Natural m.c. Depth to rigid layer	3,3 3,3 3,3 3,3
11				

TEST

Material Tested	SHRP DESIG C.I.	SHRP prof. C.I.	Method	Other
Asphalt Concrete Max. Spec. G. A.C. Content (Extenu) Extenu	AC03 AC04	P03 P04	AASHTO T209-82 AASHTO T164-86	"
Extracted Aggregate Type and Class Coarse Aggregate Roundness Index of Fine Aggregate NAA + St. F. Agg. Part. Shape Graduation	AG03 AG03 AG03 AG05 AG04	AG03 AG03 PIYA PIYA	ASTM 2488-84 ASTM 2488-84 NAATRI AASHTO T30-87I	Natl. Assoc. perfumed no cost to st. No Information Natl. Assoc. Assoc. perfumed no cost to state
Asphalt Cement Asphalt Recovery Pen. 50, 77, 90°F Spec. Grav. 60°F Viscosity @ 77°F Viscosity @ 140, 275 AE06	AE01 AE02 AE04 - -	AE01 AE02 AE04 - -	AASHTO T170-84 AASHTO T 44-87 I AASHTO T 228-87 I ASTM D3205-86 AASHTO T202-84	C.O. Lab " can't do pen @ 50 " " " " "

TEST

Material Tested

~~Sample
No.~~
~~Test No.~~

Method

Other

Sample No.	Details	Method	TEST	
			T ₁	T ₂
(1) Unbound Base & Sub-Bases				
Particle size -	U.G01	P41	AASHTO T27-88I	
Sieve Analysis (wet wt.)	U.G02	P41	AASHTO T11-85	
Atterbergs Limit	U.G04	P43	AASHTO T84-87I	
Classification	SS04	P52	AASHTO M145-82/	
Moisture Density	SS05	P44	ASTM D2488-84/	
1 Resilient Mod'ly	U.G07	P46	AASHTO T49-86/	
Classification	U.G08	P47	AASHTO T180-86	
Permeab. I.T.	U.G09		ASTM D2488-84	
Nat. Hairs. L/G. t.R	U.G10	P49	ASTM D2434-68(9)	
Subgrade				
Soil Analysis	SS01	P51	AASHTO T27-88I	
Hydro. O. Calm	SS02	P42	AASHTO T88-86	
Atter. Limt.	SS03	P43	AASHTO T89-87I/	
Classification	SS04	P52	AASHTO M145-82/	
Moisture Density, Relation	SS05	P55	F-T-H D2488-84/	
Resilient Modulus	SS07	P46	AASHTO T49-86/	
Unit Wt.	SS08		SHRP-LTP Method	
Natural Hairs. Cont.	SS09	P49	AASHTO T265-86	

O 6" OD Core of A.C. Pavement

Augering of unstabilized Base and Subbase; Splitspoon Sampling and/or shelly to 5.0' below subgrade.

NATIONAL
30 SHEETS 3 SQUARE
300 SHEETS 30 SQUARE
3000 SHEETS 300 SQUARE

TEST	OLD	NEW	*	*
			Per cent loss	Per cent loss
A ₁	thickness, Creep, Classification, moisture content.	6, 1, 6, 6 moisture suscept., classification, unit wt., thickness	3, 3, 3	3, 3, 3
A ₂	thickness, Classification, moisture content.	6, 6, 6 moisture suscept., classification, unit wt., thickness	3, 3, 3	3, 3, 3
A ₃	" " "	" moisture suscept., classification, unit wt., thickness	3, 3, 3	3, 3, 3
A ₄	" " "			
A ₅	" " "	moisture susceptibility - see if wavy around edges or there is a difference in integrity.		
A ₆	" " "	classification; - UG08		
		Unit wt; - 5508		

UNDISTURBED

A HOLES

Material Tested	SHRP Design	SHRP Protocol	Test Sample(s) Type	Method
ASPHALT CONCRETE moisture susceptibility, Thickness			4	see if raveling around edges, visual
SUBGRADE classification unit weight moisture, content Resilient Mod.	SS04	PS2	4	AASHTO M145-82 / - need hydr. ASTM D2488-84 SHRP-LTPP Method -